TOWN OF ADDISON CHANGE ORDER FORM



Change Order Number: 2

Project Name: Addison Airport US Customs and Border

Protection Facility

Project Number(s): 19-97

Contractor Name: JC Commercial, Inc.

Date: 05/20/2020

A. <u>INTENT OF CHANGE ORDER</u>

The purpose of this change order is to provide an amendment to the construction contract for JC Commercial to include the items mentioned below.

B. DESCRIPTION OF CHANGE

Items listed in section C will increase the contract amount to include unforeseen site conditions and requests from the US Customs and Border Protection Agency.

C. REASON FOR CHANGE

The following items will be included in the construction contract:

- Additional concrete pier depths were needed for the building foundation due to a
 variable depth rock layer that was deeper than anticipated. Additional pier depths
 ranged from 1.5 to 3.5 feet in depth and occurred outside the known rock layer depth
 provided in the geotechnical report. The pricing providing by JC Commercial is
 consistent with industry standards for cost per linear foot of pier depth for the
 additional work and material materials.
- Page (Architect) and AG&E (Structural Engineer) have both reviewed the proposal submittal by JC Commercial and agree the additional steel and erection cost associated with design revisions for building modifications is justified given this steel and detailing was added to the documents to accommodate layout revisions. Based on experience from the structural engineer, the cost submitted by JC Commercial appears to be approximately double what would be expected based on the erected steel tonnage involved in the change, but since the steel was already fabricated and on-site (which incurred trip charges and refabricating costs), the cost is reasonable. The steel modification required a high degree of detail and precision considering the modification was for the elevator enclosure and the contractor is required to complete the modifications based on the tight tolerances associated with the elevator installation.
- Accepted for additional site fill, excluding the building foundation footprint, due to the demolition operations performed under a separate contract removed existing soil and material that was not replaced back to the original survey grade. The site design earthwork quantity in the bid documents was calculated utilizing the average end area method based on the existing site survey before site demolition. JC Commercial performed a topographic survey of the site to verify the existing conditions before construction began and notified the project team the elevations of the site were lower

than the existing survey provided in the design documents. Also, additional fill was approved got placement along the western edge of the building foundation to protect the foundation edge until the airside improvements can be made under a separate construction contract executed by TxDOT Aviation.

 As requested by Customs and Border Protection, hold room toilets and lavatory were revised from separate toilets and lavatory to combination toilet/lavatory fixtures.

D. <u>EFFECT OF CHANGE ON CONTRACT PRICE</u>

This change order will have the following effect on the cost of this project:

Item Number/Description	Amount
Additional Foundation Pier Depths	\$11,466.65
Changes to Elevator Steel Structure	\$20,067.30
Additional Fill (Civil)	\$107,056.95
CBP Hold Room Combination Toilet/Lavatory Revision	\$3,290.20
Subtotal	\$141,881.10
Amount of this Change Order	\$141,881.10
Original Contract Amount	\$6,223,949.00
Overall Contract Amount (Including Previous Change Orders)	\$6,246,209.16
Revised Contract Amount	\$6,388,090.26
Total % Increase/Decrease (Including Previous Change Orders)	2.57%

E. **EFFECT OF CHANGE ON CONTRACT TIME**

The work required under this change will affect 25 days of the contract time.

Item Number/Description	Calendar Days
Additional Foundation Pier Depths	5
Changes to Elevator Steel Structure	10
Additional Fill (Civil)	10
Subtotal	25
Original Contract Start Date	September 30, 2019
Original Contract Time	450
Previously Approved Contract Time Extensions	0
Approved Contract Time Extensions this Change Order	25
New Contract Time	475
New Contract Completion Date	January 17, 2021

F. <u>AGREEMENT</u>

By the signatures below, duly authorized agent of the Town of Addison, and JC Commercial, Inc, do hereby agree to append this Change Order Number 2 to the original contract between themselves, dated September 18, 2019.

City State Zip	
Phone: 972-436-4622	Project Manager
Contractor's Signature	Department Director
Mills (14) Engineer's Signature Digitally Signed 05/29/2020	Fin. & Strat. Services Representative
	City Manager
Copies: Contractor (2) Department City Secretary	Council Agenda: Agenda Date (if applicable) Item Number



RFI Response

RFI ID	082		
То	Trevor Fitzgerald JC Commercial, Inc. 1801 Lakepointe Drive Suite 129 Lewisville TX 75057 United States (972) 436-4622	From Will Butler Page Southerland 1800 Main Street Suite 123 Dallas TX 75201 United States (469) 621 4837	
Project	417151 - City of Addison Airport	Project No.	417151
Date	4/8/2020	Transmittal ID	00645
Subject	Proposal #11 - Additional Pier Depths		
We are sending	☐ Attached☐ Under Separate Cover	Via	Info Exchange

Question:

Please see the attached proposal. I apologize that these proposals are out of order. The previous project engineer skipped over #11. Going forward this will not be an issue.

Suggestion:

Answer:

Response (Answered) from: Will Butler (Page Southerland Page, Inc.)

Remarks

The proposal as submitted is reasonable and acceptable. We will work to include the proposal value indicated here in the next change order.

Contents

Copies	Date	Number	Description	
1	3/3/2020		RFI 82 - Proposa (combined).pdf	al #11 - Additional Pier Depths
nese are ansmitted	☐ For review a	and comment	☐ For your use	☐ As requested

Please let me know if you need any additional information or clarification. Thank you.

CC:

Darci Neuzil Jeff Mechlem Joe McAnally Joel Jenkinson Lisa Pyles

Margarita de Monterrosa

Michael Haskins Michelle LeBlanc Scott Arthur Will Butler



Request for Information

082

Project Title:

Submitted by Subcontractor:

Addison Airport Customs and Border Protection Facility

JC Commercial

Category:

Date Created: 3/3/2020

Concrete

Answer Company
Author Company
Authored By
Page Southerland Page, Inc.
JC Commercial
Trevor Fitzgerald
1100 Louisiana, Suite One
1801 Lakepointe Dr. Suite 129
Houston, Texas 77002
Lewisville, TX 75057

Subject:

Proposal #11 - Additional Pier Depths

Question: Date Required: 3/10/2020

Please review the attached proposal for changes to the contract based on additional certified pier depths from the Fugro testing company.

Answer: Date Answered:



Project:	Addison Airport Customs and		Customer	lown of Addison		
	Border Protection Facility			5350 Beltline Road		
Date:	3/3/2020			Dallas, Texas 7525	4	
Attn:	Jeff Mechlem, Mitchell McAnally					
	Will Butler, Vlad Stevanovic			1100 Louisiana Str		
				Houston, Texas 77		
	osal contains changes in the Contract Sum and/or Con certified pier depths from testing company (Fugro).	tract Time in resp	ponse to pro	posed modifications to the	e Contract Documents	
Item	1	Description	on			Total
	Proposal #11 - Additional Pier Depths					\$ 9,130.95
		-			Sub-total	\$ 9,130.95
					General Conditions	\$ 456.55
					Sub-total	
					Insurance	
					Bond	
					Sub-total	\$ 9,971.00
					Mark up	
					Total	\$ 11,466.65
	posed Change involve a change in Contract Sum?	[] No	[X] Yes	[Increase]		\$11,466.65
Does Pro	posed Change involve a change in Contract Time?	[] No	[X] Yes 	[Increase]	Days _.	5
JC Comm	rercial			Page Southerland Page		
CONTRA				Architect		·
	epointe Drive, Suite #129 , TX 75057			1100 Louisiana Street, S Houston, Texas 77002	uite One	
Ву:	Scott Arthur			Ву:		
Signatu	ıre: Scott Arthur			Signature:		
Date:	3/3/2020			Date:		

Proposal #11

Pier Depths

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

G-2	F-	7	E-7	A-7	A.5-7.2	A-6	A.5-6.2
3	16	36	36	24	24	24	24
	4	14	14	11	11	11	11
0	.9	0.3	0	0.5	0.5	0.7	1
99	.5	99.5	99.5	99.5	99.5	99.5	99.5
9)7	97	97	97	97	97	97
14	.6	14.4	13.1	14	13.6	14.3	14
0	.2	0.6	-0.4	3	2.6	3.1	2.5

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

H-7		H-6	F.8-6	F-6	E.7-5.1	D-5.2	C-5.2
	24	24	24	24	24	24	24
	11	11	11	11	11	11	11
	0.8	0.6	2.7	2.7	0.6	0.7	0.6
	99.5	99.5	99.5	99.5	99.5	99.5	99.5
	97	97	97	97	97	97	97
	15.6	15	13.7	13.8	11.6	14.6	14.1
	4.3	3.9	0.5	0.6	0.5	3.4	3

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

D-3		E-5	E.7-5.7	D.4-5.4	D-2	E.7-2	E-3
	24	24	24	24	24	24	24
	11	11	11	11	11	11	11
	0.9	0.4	0.3	0.4	0.4	0.5	0.6
	99.5	99.5	99.5	99.5	99.5	99.5	99.5
	97	97	97	97	97	97	97
	15.1	15	11.3	14.5	14.3	14.2	14.8
	3.7	4.1	0.5	3.6	3.4	3.2	3.7

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

C-4		E.6-6	F.8-6.5	F-6.5	D-6	D-7	G.5-D.5
	24	24	24	24	24	36	18
	11	11	11	11	11	14	11
	2.4	0.5	0.9	0.7	0	0	0.2
	99.5	99.5	99.5	99.5	99.5	99.5	99.5
	97	97	97	97	97	97	97
	13.5	14.3	16.2	14.2	14	15.2	11.5
	0.6	3.3	4.8	3	3.5	1.7	0.8

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

D.8-0.5	C.8-D.5	H-1	G.5-0.5	G-0.8	G.2-2.5	A-5
18	18	24	24	24	24	24
11	11	11	11	11	11	11
0	0.1	0.1	0	0	0	0.1
99.5	99.5	99.5	99.5	99.5	99.5	99.5
97	97	97	97	97	97	97
12.6	14.1	11.6	11.6	11.6	11.5	14
2.1	3.5	1	1.1	1.1	1	3.4

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

B-2.3	A.9-4.4	A-1	C-1	C-2	E-1	B-3
24	1 24	36	36	36	36	36
10	11	14	14	14	14	14
0.3	0.1	0	0	0.1	0.3	0.1
99.	99.5	99.5	99.5	99.5	99.5	99.5
97	7 97	97	97	97	97	97
13.6	13.5	16.8	16	16.1	14.9	15.1
3	3 2.9	3.3	2.5	2.5	1.1	1.5

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

B-4		B-5.2	F-1	G-3	G-5	G-2	F-7
	_36	36	36	36	36	36	36
	14	14	14	14	14	14	14
	0	0	0.2	0	0.3	0.9	0.3
9	9.5	99.5	99.5	99.5	99.5	99.5	99.5
	97	97	97	97	97	97	97
	15	15	14.2	13.7	13.8	14.6	14.1
	1.5	1.5	0.5	0.2	0	0.2	0.3

Piers
Pier Size
Estimated Depth
Over Drilled Depth from Driller
Ground Surface Elevation
Top Of Shaft Elevation
Total Drilled Shaft Depth
Additional Depth of Pier

E-7	A-7		A.5-7.2	A-6	A.5-6.2	
3	6	24	24	24	24	
1	4	11	11	11	11	
	0	0.5	0.5	0.7	1	
99.	5	99.5	99.5	99.5	99.5	
9	7	97	97	97	97	
13	1	14	13.6	14.3	14	
-0.	4	3	2.6	3.1	2.5	

Total Additional LF		
112 70'	\$ 81.02	_

Grand Total: \$9130.95



Fugro USA Land, Inc.

2880 Virgo Lane

Dallas, Texas 75229

Phone (972) 484-8301, Fax (972) 620-7328

DAILY FIELD SUMMARY REPORT

Project: Client:

Addison Airport CBPF
Town of Addison

16801 Westgrove Dr. Addison, TX 75001

Service Date:

11/13/2019

Report Date:

11/14/2019

Project No.:

04.40192101

Lab / Report No.:

21141-1 / 0023

Page 1 of 4

Summary of Field Activities and Observations

On this date the representative of Fugro USA Land, Inc. noted below was present at the project site to perform services as scheduled.

Pier foundation installation observation services were performed. A total of 18 piers were installed. See the attached "Drilled Pier Observation Report" for additional details.

Fugro Representative:

James Pacheco: Left for Job: 6:30 am: Arrive: 7:00 am Depart:

5:00 pm

Travel: 1.0 Total Billable Hours: R/T: 11.0

Fugro USA Land, Inc.

TBPE Firm Registration No. F-299

Muhammad Khan Project Manager



3

2880 Virgo Lane

Dallas, TX 75229

Phone: (972) 484-8301 Work Order#21141 Rpt#0023

DRILLED PIER OBSERVATION REPORT

Client:

Town of Addison

11/13/19

Project No: 04.40192101

Project: Date:

Addison Airport

Drilling Firm: Maxon Drilling

Page

of

				-	, age	1	. 01	
Pier Identification: Buil	ding Pad	G-2	F-7	E-7	A-7	A.5-7.2	A-6	A.5-6.2
Pier Diameter, in.	Required	36	36	36	24	24	24	24
Pier Diameter, in.	Actual	36	36	36	24	24	24	24
Time Drilling Started		8:00	8:25	8:58	9:20	9:37	9:49	10:03
Top of Ground Elevation	on, ft. ⁽¹⁾	99.8	99.8	99.8	99.8	99.8	99.8	99.8
Top of Pier Elevation, f		97.0	97.0	97.0	97.0	97.0	97.0	97.0
Required Depth, ft.		8.70	8.80	8.10	10.50	10.10	10.60	10.00
Total Depth, ft.		14.60	14.10	13.10	14.00	13.60	14.30	14.00
Peneration ft.	Required	5.00	5.00	5.00	5.00	5.00	5.00	5.00
	Actual	5.90	5.30	5.00	3.50	3.50	3.70	4.00
Casing	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Plumb within Tolerance	(Yes/No):	YES	YES	YES	YES	YES	YES	YES
	Bar Size	#8	#8	#8	#6	#6	#6	#6
Vertical Reinforcing	Quantity	12	12	12	6	6	6	6
	Length, ft	11.70	11.20	10.20	11.10	10.70	11.40	11.10
Horizontal Reinforcing	Bar Size	#3	#3	#3	#3	#3	#3	#3
Horizontal Reinforcing Spacing, in		12	12	12	12	12	12	12
Time Concrete Placed		11:40	11:40	11:42	11:43	11:46	11:48	13:20
Condition of Bottom be Placement (Wet / Drv)	fore Concrete	DRY	DRY	DRY	DRY	DRY	DRY	DRY
Time Drilling Complete	d	8:24	8:57	9:19	9:36	9:48	10:02	10:17

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

Work Order#21141 Rpt#0023

DRILLED PIER OBSERVATION REPORT

Client:

Town of Addison

Project No: 04.40192101

Project: Date: Addison Airport

Drilling Firm: Maxon Drilling

Page

2 of 3

				•			•	
Pier Identification: Buil	ding Pad	H-7	H-6	** F.8-6	** F-6	E.7-5.1	D-5.2	C-5.2
Pier Diameter, in.	Required	24	24	24	24	24	24	24
riei Diameter, in.	Actual	24	24	24	24	24	24	24
Time Drilling Started		10:18	10:38	10:53	11:05	11:22	11:37	11:50
Top of Ground Elevation	n, ft. ⁽¹⁾	99.8	99.8	99.8	99.8	99.8	99.8	99.8
Top of Pier Elevation, ft		97.0	97.0	93.6	93.6	97.0	97.0	97.0
Required Depth, ft.		11.80	11.40	8.00	8.10	8.00	10.90	10.50
Total Depth, ft.		15.60	15.00	13.70	13.80	11.60	14.60	14.10
Peneration ft.	Required	3.00	3.00	3.00	3.00	3.00	3.00	3.00
reneration it.	Actual	3.80	3.60	5.70	5.70	3.60	3.70	3.60
Casing	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Plumb within Tolerance	(Yes/No):	YES	YES	YES	YES	YES	YES	YES
	Bar Size	#6	#6	#6	#6	#6	#6	#6
Vertical Reinforcing	Quantity	6	6	6	6	6	6	6
	Length, ft	12.70	12.10	7.40	7.50	8.70	11.70	11.20
Horizontal Reinforcing	Bar Size	#3	#3	#3	#3	#3	#3	#3
Tionzontal Neimorchig	Spacing, in	12	12	12	12	12	12	12
Time Concrete Placed		13:30	13:35	13:38	13:43	13:50	15:10	15:05
Condition of Bottom before Concrete Placement (Wet / Drv)		DRY	DRY	DRY	DRY	DRY	DRY	DRY
Time Drilling Completed	i	10:37	10:52	11:04	11:21	11:36	11:49	12:03

Remarks: Piers F.8-6 & F-6 are for Elevator Shaft. Due to elevation of top of pier, had to drill deeper to achieve min per length.

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

Work Order#21141 Rpt#0023

DRILLED PIER OBSERVATION REPORT

Client:	Town of Addison	Project No:	04.4019210)1	
Project:	Addison Airport	Drilling Firm:	Maxon Drill	ing	
Date:	11/13/19	Page	3	of	3
				,	

Pier Identification: B	uilding Pad	D-3	E-5	E.7-5.7	D.4-5.4			
Pier Diameter, in.	Required	24	24	18	18_			
The Diameter, III.	Actual	24	24	18	18			
Time Drilling Started		12:45	13:00	13:10	13:50			
Top of Ground Eleva	tion, ft. (1)	99.8	99.8	99.8	99.8			
Top of Pier Elevation	, ft	97.0	97.0	97.0	97.0			
Required Depth, ft.		11.20	11,60	8.00	11.10			
Total Depth, ft.		15.10	15.00	11.30	14.50			
Peneration ft.	Required	3.00	3.00	3.00	3.00			
r eneration it.	Actual	3.90	3.40	3.30	3.40			
Casing	Dia., in.	N/A	N/A	N/A	N/A			
	Length, ft	N/A	N/A	N/A	N/A			
Plumb within Tolerar	nce (Yes/No):	YES	YES	YES	YES			
	Bar Size	#6	#6	#6	#6			
Vertical Reinforcing	Quantity	12	12	12	12		1	
	Length, ft	12.20	12.10	8.40	11.60			
Horizontal	Bar Size	#3	#3	#3	#3			
Reinforcing	Spacing, in	12	12	12	12	<u>"</u>	1	
Time Concrete Placed		12:59	13:05	13 22	14:05			
Condition of Bottom before Concrete Placement (Wet / Drv)		DRY	DRY	DRY	DRY			
Time Drilling Comple		15:15	15:25	15:30	15:40			

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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Fugro USA Land, Inc.

2880 Virgo Lane Dallas, Texas 75229

Phone (972) 484-8301, Fax (972) 620-7328

DAILY FIELD SUMMARY REPORT

Project: Client: Addison Airport CBPF
Town of Addison

16801 Westgrove Dr. Addison, TX 75001 Service Date:

11/14/2019

Report Date:

11/25/2019

Project No.:

04.40192101

Lab / Report No.:

21142-1 / 0027

Page 1 of 3

Summary of Field Activities and Observations

On this date the representative of Fugro USA Land, Inc. noted below was present at the project site to perform services as scheduled.

Pier foundation installation observation services were performed. A total of 9 piers were installed. See the attached "Drilled Pier Observation Report" for additional details.

Fugro Representative:

James Pacheco: Left for Job: 6:30 am : Arrive: 7:00 am Depart:

4:00 pm

Travel: 1.0 Total Billable Hours: R/T: 10.0

Fugro USA Land, Inc.

TBPE Firm Registration No. F-299

Muhammad Khan Project Manager

THE ABOVE TEST RESULTS APPLY ONLY TO THE ITEMS TESTED.
THIS REPORT SHALL NOT BE REPRODUCED EXCEPT IN FULL WITHOUT THE APPROVAL OF FUGRO USA LAND, INC.



2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

Work Order#21142 Rpt#0027

DRILLED PIER OBSERVATION REPORT

 Client:
 Town of Addison
 Project No: 04.40192101

 Project:
 Addison Airport
 Drilling Firm: Maxon Drilling

 Date:
 11/14/19
 Page 1 of 2

				-	J	<u>'</u>	-	
Pier Identification: B	uilding Pad	D-2	E.7-2	E-3	C-4	E.6-6	*F.8-6.5	*F-6.5
Pier Diameter, in	Required	24	24	24	24	24	24	24
Pier Diameter, in. Time Drilling Started Top of Ground Eleva Top of Pier Elevation Required Depth, ft. Total Depth, ft. Teneration ft. Ten	Actual	24	24	24	24	24	24	24
Time Drilling Started		8:00	8:26	8:43	9:10	9:27	9:49	10:19
Top of Ground Elevat	tion, ft. ⁽¹⁾	99.8	99.8	99.8	99.8	99.8	99.8	99.80
Top of Pier Elevation,	ft	97.0	97.0	97.0	97.0	97.0	91.6	93.60
Required Depth, ft.		10.90	10.70	11.20	10.00	8.20	8.40	8.40
Total Depth, ft.		14.30	14.20	14.80	13.50	14.30	16.20	14.20
Peneration ft	Required	3.00	3.00	3.00	3.00	3.00	3.00	3.00
	Actual	3.40	3.50	3.60	3.50	6.10	7.80	5.80
Casino	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Plumb within Toleran	ce (Yes/No):	YES	YES	YES	YES	YES	YES	YES
	Bar Size	#6	#6	#6	#6	#6	#6	#6
Vertical Reinforcing	Quantity	6	6	6	6	6	6	6
	Length, ft	11.40	11.30	11.90	10.60	11.40	7.90	7.90
Horizontal	Bar Size	#3	#3	#3	#3	#3	#3	#3
Reinforcing	Spacing, in	12	12	12	12	12	12	12
Time Concrete Placed		8:25	8:42	9:09	9:26	9:48	10:18	10:37
Condition of Bottom before Concrete Placement (Wet / Drv)		DRY	DRY	DRY	DRY	DRY	DRY	DRY
Time Drilling Comple		15:10	15:15	15:22	15:31	13:41	13:32	13:28

Remarks:

*Piers F.8-6.5 & F-6.5, Due to top of pier elevation, we had to drill deeper to achieve min pier length.

Tech:

James Pacheco

FUGRO USA Land, INC.
TBPE Firm Registration No. F-299

Muhammad Khan, P.E. Project Manager

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2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

Work Order#21142 Rpt#0027

DRILLED PIER OBSERVATION REPORT

Client:	Town of Add	dison				Project No: 04.40192101					
Project:	Addison Air	port			-	Drilling Firm:					
Date:	11/14/19				- -	Page		_ of	2		
Pier Identi	ification: Buil	ding Pad	D-6	D-7				1			
Pier Diam	otos in	Required	24	24	 			 -			
PIEL DIAITS	eter, in.	Actual	24	24							
Time Drilli	ng Started		10:30	10:55							
Top of Gro	ound Elevation	n, ft. ⁽¹⁾	99.8	99.8							
Top of Pie	r Elevation, ft		97.0	97.0							
Required I	Depth, ft.		11.00	10.20							
Total Dept	th, ft.		14.00	15.20				2			
Peneration	. f	Required	3.00	5.00					 		
i cheratioi		Actual	3.00	5.00					1		
Casing		Dla., in.	N/A	N/A							
		Length, ft	N/A	N/A							
Plumb with	hin Tolerance	(Yes/No):	YES	YES							
		Bar Size	#6	#8		1		 			
Vertical Re	inforcing	Quantity	6	12							
		Length, ft	11.10	12.30							
Horizontal	Reinforcing	Bar Size	#3	#3							
i iorizoritai		Spacing, in	12	12			-				
	rete Placed		10:50	11:15							
	of Bottom be (Wet / Drv)	ore Concrete	DRY	DRY							
	ng Completed	I	13:20	13:15							

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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Fugro USA Land, Inc.

2880 Virgo Lane Dallas, Texas 75229 Phone (972) 484-8301, Fax (972) 620-7328

DAILY FIELD SUMMARY REPORT

Project: Client: Addison Airport CBPF

Town of Addison 16801 Westgrove Dr. Addison, TX 75001 **Service Date:**

11/06/2019

Report Date:

11/13/2019

Project No.:

04.40192101

Lab / Report No.:

21055-1 / 0020

Page 1 of 4

Summary of Field Activities and Observations

On this date the representative of Fugro USA Land, Inc. noted below was present at the project site to perform services as scheduled.

Pier foundation installation observation services were performed. A total of 20 piers were installed. See the attached "Drilled Pier Observation Report" for additional details.

Fugro Representative:

James Pacheco: Left for Job: 6:30 am : Arrive: 7:00 am Depart:

5:00 pm

Travel: 1.0 Total Billable Hours: R/T: 11.0

Fugro USA Land, Inc.

TBPE Firm Registration No. F-299

Muhammad Khar Project Manager



2880 Virgo Lane

Dallas, TX 75229

Phone: (972) 484-8301 Work Order#21055 Rpt#0020

DRILLED PIER OBSERVATION REPORT

Client: Project:

Date:

Town of Addison

Addison Airport

11/06/19

Project No: 04.40192101

Drilling Firm: Maxon Drilling Page

3

				•	· ugu		. 01	
Pier Identification: Build	ding Pad	G.5-D,5	D.8-O.5	C.8-D,5	H-1	G,5-O,5	G-0.8	G.2-2.5
Pier Diameter, in.	Required	18	18	18	24	24	24	24
riei Diametei, in.	Actual	18	18	18	30	30	24	24
Time Drilling Started		8:00	8:30	8:51	9:08	9:23	9:37	9:56
Top of Ground Elevatio	n, ft. ⁽¹⁾	99.8	99.8	99.8	99.8	99.8	99.8	99.8
Top of Pier Elevation, ft		97.0	97.0	97.0	97.0	97.0	97.0	97.0
Required Depth, ft.		8.30	9.60	11.00	8.50	8.60	8.60	8.50
Total Depth, ft.	-···	11.50	12.60	14.10	11.60	11.60	11.60	11.50
Peneration ft.	Required	3.00	3.00	3.00	3.00	3.00	3.00	3.00
	Actual	3.20	3.00	3.10	3.10	3.00	3.00	3.00
Casing	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	N/A
	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Plumb within Tolerance	(Yes/No):	YES	YES	YES	YES	YES	YES	YES
	Bar Size	#6	#6	#6	#6	#6	#6	#6
Vertical Reinforcing	Quantity	5	5	5	6	6	6	6
	Length, ft	8.60	9.70	11.20	8.70	8.70	8.70	8.60
Horizontal Reinforcing	Bar Size	#3	#3	#3	#3	#3	#3	#3
Spacing, in		12	12	12	12	12	12	12
Time Drilling Completed		8:29	8:50	9:07	9:22	9:36	9:55	10:13
Condition of Bottom before Concrete Placement (Wet / Drv)		DRY	DRY	DRY	DRY	DRY	DRY	DRY
Time Concrete Placed		12:30	12:34	12:45	12:56	13:01	13:08	13:15

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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Fugro Dallas 2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

DRILLED PIER OBSERVATION REPORT

Client:

Town of Addison

11/06/19

Project No: 04.40192101

Project: Date: Addison Airport

Drilling Firm: Maxon Drilling

Page 2 of 3

Work Order#21055 Rpt#0020

Pier Identification: Buil	A-2	B-2.3	A.9-4.4	A-1	C-1	C-2	E-1	
Diar Diameter in	Required Required		24	24	36	36	36	36
Pier Diameter, in.	Actual	24	24	24	36	36	36	36
Time Drilling Started		10:14	10:30	10:50	11:09	11:34	12:03	12:39
Top of Ground Elevation	ո, ft. ⁽¹⁾	99.8	99.8	99.8	99.8	99.8	99.8	99.8
Top of Pier Elevation, ft		97.0	97.0	97.0	97.0	97.0	97.0	97.0
Required Depth, ft.	Required Depth, ft.		10.50	10.40	11.80	11.00	11.00	9.60
Total Depth, ft.	otal Depth, ft.		13.60	13.50	16.80	16.00	16.10	14.90
Peneration ft.	Required	3.00	3.00	3.00	5.00	5.00	5.00	5.00
T CHCIGGOTTE.	Actual	3.00	3.10	3.10	5.00	5.00	5.10	5.30
Casing	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	N/A
casing	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Plumb within Tolerance	(Yes/No):	YES	YES	YES	YES	YES	YES	YES
	Bar Size	#6	#6	#6	#8	#8	#8	#8
Vertical Reinforcing	Quantity	6	6	6	12	12	12	12
	Length, ft	11.10	10.70	10.60	13.90	13.10	13.20	12.00
Horizontal Reinforcing	Bar Size	#3	#3	#3	#3	#3	#3	#3
	Spacing, in	12	12	12	12	12	12	12
Time Drilling Completed		10:29	10:49	11:03	11:33	12:02	12:38	13:05
Condition of Bottom be Placement (Wet / Dry)	fore Concrete	DRY	DRY	DRY	DRY	DRY	DRY	DRY
Time Concrete Placed		13:21	13:28	13:36	15:50	15:56	16:05	16:13

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E.

Project Manager

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2880 Virgo Lane Dallas, TX 75229

Phone: (972) 484-8301

Work Order#21055 Rpt#0020

DRILLED PIER OBSERVATION REPORT

Client:

Town of Addison

Project No: 04.40192101

Project: Date:

Addison Airport

Drilling Firm: Maxon Drilling

Date: 11/06/19		-						
Date. 11/00/19				-	Page	3	of -	3
Pier Identification: B	uilding Pad	B-3	B-4	B-5.2	F-1	G-3	G-5	
Pier Diameter, in.	Required	36	36	36	36	36	36	
rici bidineter, in.	Actual	36	36	36	36	36	36	
Time Drilling Started		13:06	13:32	13:56	14:23	14:52	15;23	
Top of Ground Eleval	tion, ft. ⁽¹⁾	99.8	99,8	99.8	99.8	99.8	99.8	
Top of Pier Elevation,	. ft	97.0	97.0	97.0	97.0	97.0	97.0	
Required Depth, ft.		10.00	10.00	10,00	9.00	8.70	8.50	
Total Depth, ft.		15.10	15.00	15,00	14.20	13.70	13.80	
Peneration ft.	Required	5.00	5.00	5.00	5.00	5.00	5.00	
	Actual	5.10	5.00	5.00	5.20	5.00	5.30	
Casing	Dia., in.	N/A	N/A	N/A	N/A	N/A	N/A	
	Length, ft	N/A	N/A	N/A	N/A	N/A	N/A	
Plumb within Toleran	ce (Yes/No):	YES	YES	YES	YES	YES	YES	
	Bar Size	#8	#8	#8	#8	#8	#8	
Vertical Reinforcing	Quantity	12	12	12	12	12	12	
	Length, ft	12.20	14.10	12.10	11.30	10.80	10.90	
Horizontal	Bar Size	#3	#3	#3	#3	#3	#3	
Reinforcing	Spacing,	12	12	12	12	12	12	
Time Drilling Comple	1	13:31	13:55	14:24	14:51	15:22	15:56	
Condition of Bottom Concrete Placement (DRY	DRY	DRY	DRY	DRY	DRY	
Time Concrete Placed		16:22	16:29	16:40	16:47	16:59	17:05	

Remarks:

Tech:

James Pacheco

FUGRO USA Land, INC. TBPE Firm Registration No. F-299

Muhammad Khan, P.E. **Project Manager**

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RFI Response

RFI ID	087		
То	Trevor Fitzgerald JC Commercial, Inc. 1801 Lakepointe Drive Suite 129 Lewisville TX 75057 United States (972) 436-4622	From	Page CA Department Page Southerland Page, Inc. 1100 Louisiana Suite One Houston TX 77002 United States (713) 871-8484
Project	417151 - City of Addison Airport	Project No.	417151
Date	4/24/2020	Transmittal ID	00676
Subject	Proposal #10 - Changes to the Elevator		
We are sending	☐ Attached ☐ Under Separate Cover	Via	Info Exchange

Question:

Per the response to RFI #70, we are resubmitting proposal #10 with the following information as justification for the pricing:

It is very difficult to erect straight enough and plum enough to work with the mechanics of the car. The modifications that came out affected a number of other pieces. Four of these were already on site and had to be picked up and brought back to the shop to be refitted and then delivered once again. In regards to the labor price, the subcontractor uses his shop foreman and best fitter welders to install these items not his regular iron working crew. These guys make considerably more than the normal journeyman iron worker.

Suggestion:

Answer:

Response (Answered) from: Will Butler (Page Southerland Page, Inc.)
Remarks:

Page and AG&E have both reviewed the proposal and agree that the added steel and erection cost request is justified given when this steel and detailing was added to the documents. We do believe that the GC is owed some costs for this work and materials. Based on AG&E past experience, the cost submitted appears to be approximately double what would be expected based on the erected steel tonnage involved in the change. Upon further review and discussions with the GC and subcontractor, they have refused to reduce these costs and again referenced the fact that modifications had to be made to steel that was already fabricated and on-site (which incurred trip charges and re-fabricating costs). They also pointed out that a high degree of detail and precision is required when doing this type of elevator work due to tight elevator tolerances and because of this they have high costs for the



labor to complete this work.

We will include the submitted cost proposal value with the next change order (CO #2) along with our response above for Owner review and comment.

Contents

Copies	Date	Number					
1	3/19/2020		RFI 87 - Proposal #10 - Changes to the Elevator.pdf				
These are transmitted	☐ For review	v and comment	☐ For your use	☐ As requested			

Please let me know if you need any additional information or clarification. Thank you.

CC:

Darci Neuzil Jeff Mechlem Joe McAnally Joel Jenkinson Lisa Pyles

Margarita de Monterrosa

Michelle LeBlanc Scott Arthur Will Butler



Request for Information 87

Project Title:

Submitted by Subcontractor:

Addison Airport Customs and Border Protection Facility

Date Created:

3/19/2020

Answer Company
Page Southerland Page, Inc.
1100 Louisiana, Suite One

Houston, Texas 77002

JC Commercial

Authored By
Trevor Fitzgerald

1801 Lakepointe Dr. Suite 129

Lewisville, TX 75057

Author Company

Subject:

Category:

Proposal #10 - Changes to Elevator

Structural

Question:

Date Required:

3/26/2020

Per the response to RFI #70, we are resubmitting proposal #10 with the following information as justification for the pricing:

It is very difficult to erect straight enough and plum enough to work with the mechanics of the car. The modifications that came out affected a number of other pieces. Four of these were already on site and had to be picked up and brought back to the shop to be refitted and then delivered once again. In regards to the labor price, the subcontractor uses his shop foreman and best fitter welders to install these items not his regular iron working crew. These guys make considerably more than the normal journeyman iron worker.

Answer:

Date Answered:



INC	ORPORATED				Proposal #1	0
Project:	Addison Airport Customs and		Customer:	Town of Addison		
	Border Protection Facility			5350 Beltline Road	·	
Date:	1/31/2020			Dallas, Texas 75254	·	
Attn:	Jeff Meclem, Mitchell McAnally	_	Architect:	Page Southerland Page	e	
	Will Butler, Vlad Stevanovic			1100 Louisiana Street,		
				Houston, Texas 77002		
	osal contains an itemized quotation for changes ons to the Contract Documents based on	in the Contract Sum and	d/or Contrac	t Time in response to propose	ed	
item	1	Descriptio	n	-		Total
. 1	Changes to Elevator SK1 and SK2					\$ 15,979.70
						¥ 10 01.01.0
		.				
			-			
						A 45 070 70
				_		\$ 15,979.70
				G	Seneral Conditions	
					Sub-total	\$ 16,778.69
					Insurance	\$ 335.57
					Bond	
					Sub-total	\$ 17,449.83
					Mark up	
						\$ 20,067.30
					ı otar	4 20,007.30
Does Pro	posed Change involve a change in Contract Sun	n? [] No	[X] Yes	[Increase / Decrease]		\$20,067.30
Does Pro	posed Change involve a change in Contract Tim	e? []No	[X] Yes	[Increase / Decrease]	Days	10
JC Comm	nercial					
CONTRA	CTOR	·		Architect		
	epointe Drive, Suite #129 , TX 75057				·	
Ву:	Scott Arthur			Ву:		
Signati	Ira. Scott Arthur			Signature:		
_	arc.			_ •		
Date:	1/31/2020			Date:		

Change in Work - Cost Analysis Form

Project Naine Add son Airport	Project No. 19-010
Contractor Steconco	Change No. 1
Dascription of Change Changes at alexator SK-1 and SK-2	

eans Pe		DESCRIPTION	Quantity	Unit	ast pe	r hundred we g	cost per pound	cost per es,	Total Hourly Kate	Labor	Material Total	Equipment	Subcumulati
		Leber											
		Shup Laker	54						85.00	\$4.590.00			
		Field tabor erection	90	HRS	1				39.50	\$3,555.00		0	12
-		Office heurs	8	HRS	\vdash				65.00	\$520.00	-	2	54
	\dashv												
_	-	Motorial	-		\vdash		l						
	1	tube steel, a gles, embeds	1	lot	Ş	4,272.00				*:	\$4.272.00	-	it
	\exists	Equipment	1										_
	1	forkift	1 1	Day	\$	125.00						\$125.00	
		sc ssor lift,	1 1	Day	\$	85.00						\$85.00	
		truck expense	1	day	\$	250.00]		\$250.00	
	_		0		\$	635.00						\$0.00	
	_		0		S	180.00						\$0.00	
	_		0		\$	180.00						\$0.00	
	-	Datiesty	1	Per	\$	130.00			 			\$130.00	
		Subcentractor (Detailor)											\$1,000.00
		-	0.68		\$	400.00							\$0.00
	\dashv		0	SF	\$	1.25	Par Munin	2.00					\$0.00
	_	SUBTOTAL	s		<u> </u>					\$8,665.00	\$4,272.00	\$590.00	\$1,000

Labor Equipment & Materia	I Tota)	\$ 13,527.00
Subcontractor		\$1,000.00
Overhead and Profit	10%	\$1,452.70
Taxes	0,00%	\$0.00
	TOTAL FOR THIS CHANGE	\$15,979,70

All material is founded up

Material All material has a \$130.00 Delivery Fee in DFW

Equipment

Equipment is rented by the day. Abstrning over 3 days great in 1 wees, Anything over 2 weeks great in 4 weeks.

All equipment has a pick up and delivery charge of \$130.00; This includes small equipment.



RFI Response

RFI ID	086		
То	Trevor Fitzgerald	From	Page CA Department
	JC Commercial, Inc.		Page Southerland Page, Inc.
	1801 Lakepointe Drive		1100 Louisiana
	Suite 129		Suite One
	Lewisville TX 75057		Houston TX 77002
	United States		United States
	(972) 436-4622		(713) 871-8484
Project	417151 - City of Addison Airport	Project No.	417151
Date	4/6/2020	Transmittal ID	00643
Subject	Proposal #16 - Additional Fill (Civil)	. =:-	
We are sending	☐ Attached	Via	Info Exchange
	☐ Under Separate Cover		

Question:

Please see the attached proposal.

Suggestion:

Answer:

Response (Answered) from: Vladimir Stevanovic (Garver)

Remarks:

Contents

	Copies	Date Number		Description				
	1	3/19/2020	3/19/2020		sal #16 - Additional Fill (Civil).pdf			
These are transmitted		☐ For revie	w and comment	☐ For your use	☐ As requested			

Please let me know if you need any additional information or clarification. Thank you.

CC:

Darci Neuzil Jeff Mechlem Joe McAnally Joel Jenkinson Lisa Pyles

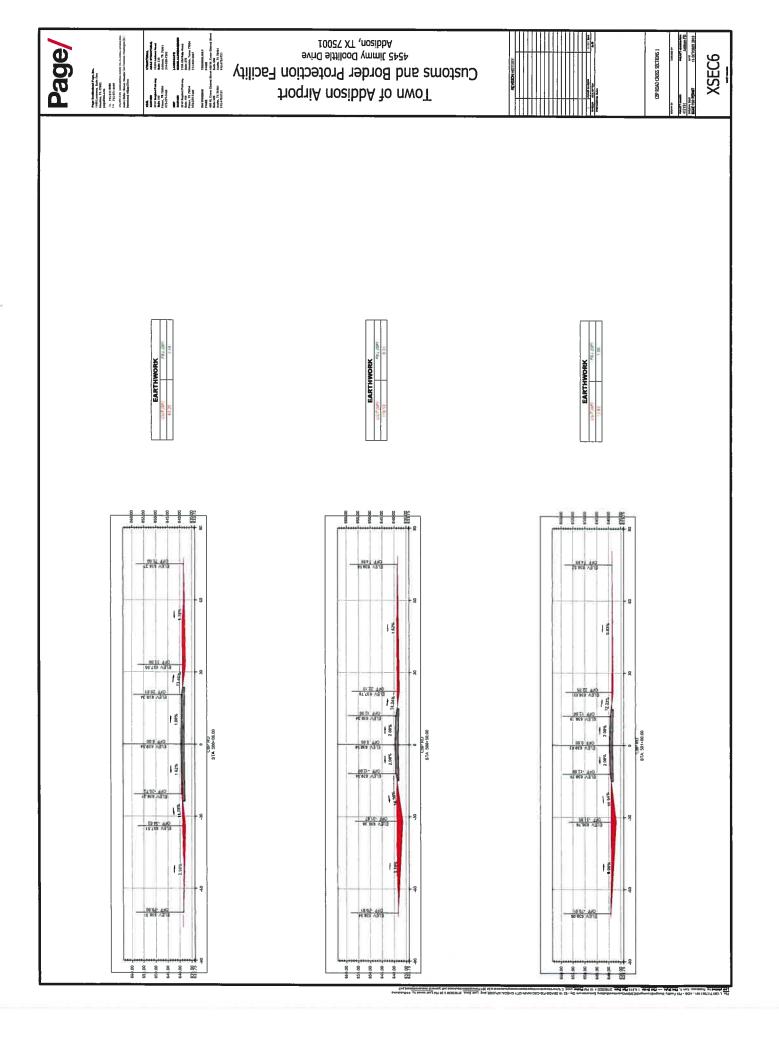
Margarita de Monterrosa

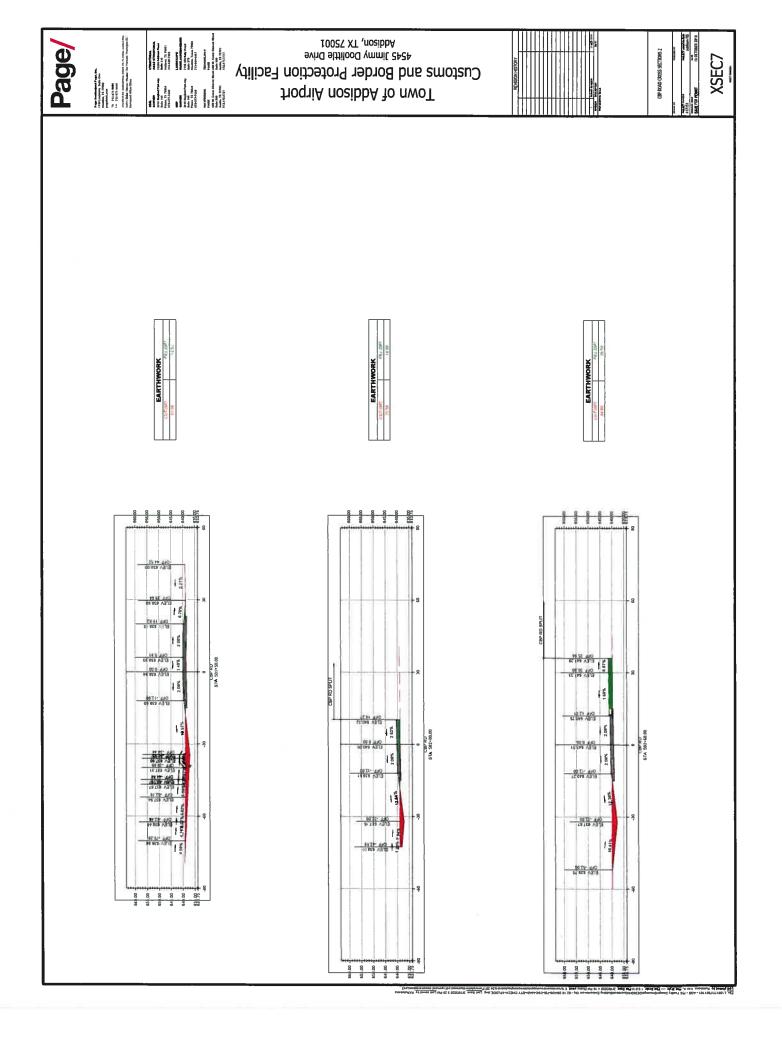
Michael Haskins Michelle LeBlanc Scott Arthur Will Butler

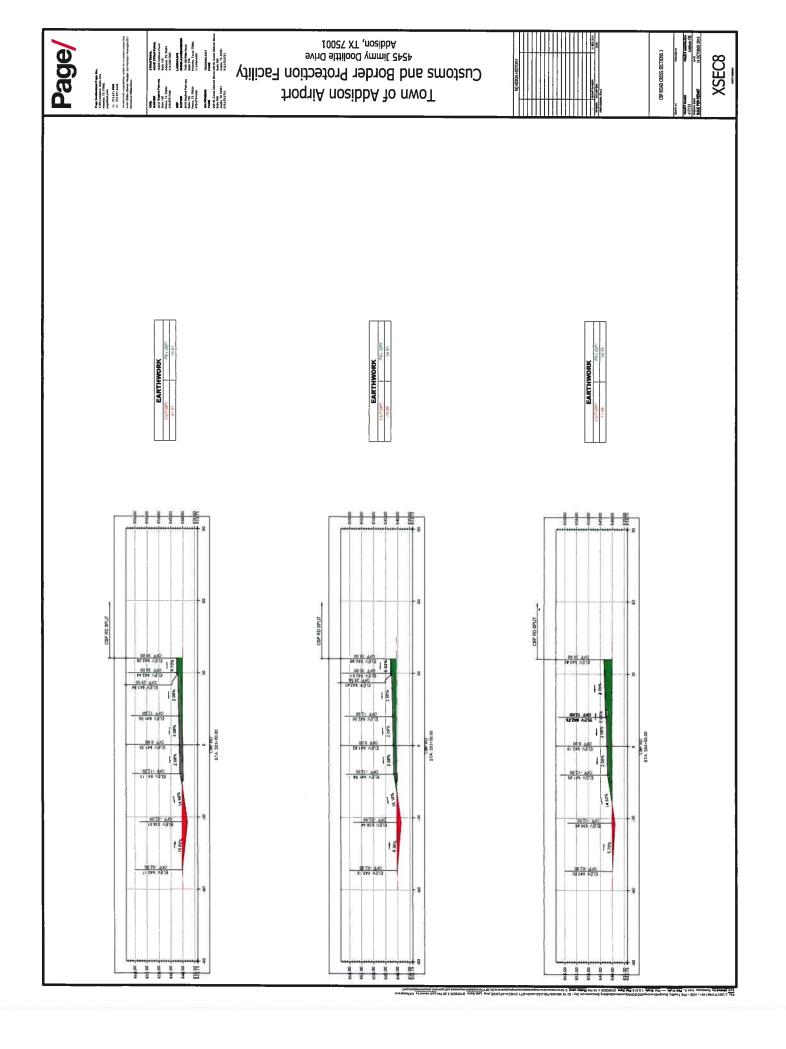


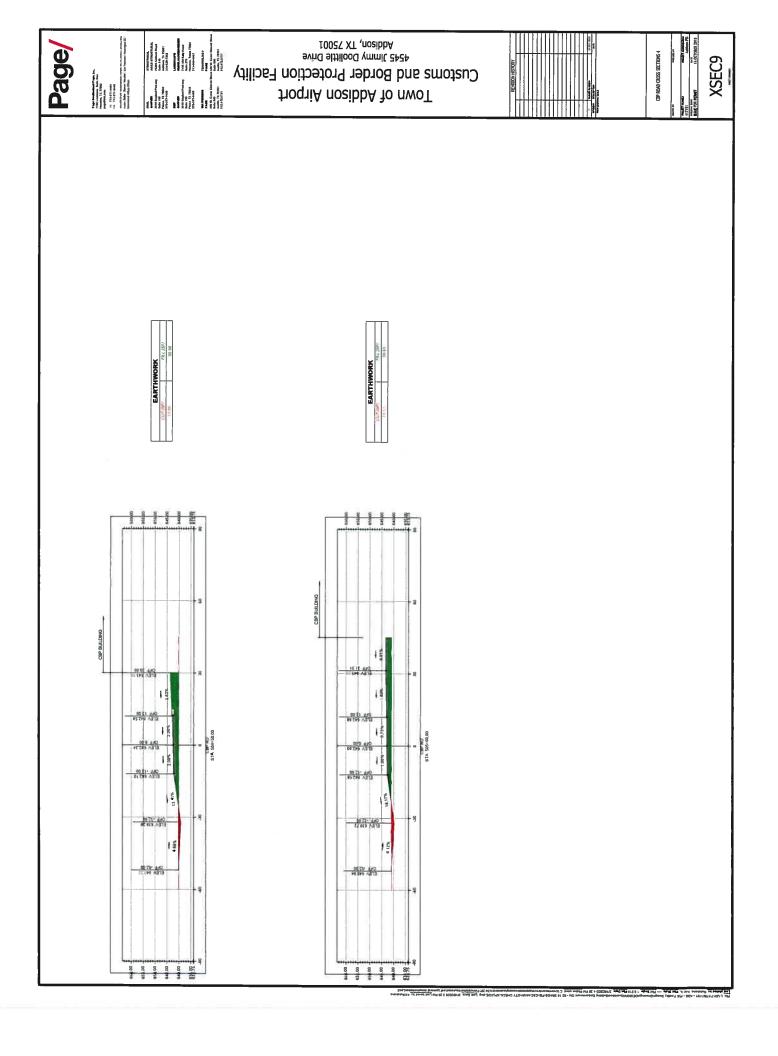
Project:	Addison Airport Customs and		Customer:	I own of Addison			
	Border Protection Facility			5350 Beltline Road			
Date:	3/19/2020			Dallas, Texas 75254			
Attn:	Jeff Mechlem, Mitchell McAnally		Architect:	Page Southerland Page			
	Will Butler, Vlad Stevanovic			1100 Louisiana Street, S	uite One		
				Houston, Texas 77002			
	osal contains a quotation for changes in the Contract						
modificati	ons to the Contract Documents based on RFI 13 and	field directive from	Garver/Pag	je.			
	1						
Item		Description	***				Total
1	1550 yards of common fill for civil area of the proje	ect only. This exclude	des the build	ing pad.		\$	85,250.00
	<u>.</u>					_	
						<u> </u>	
						_	
						<u> </u>	
		-				<u> </u>	
_					0.54-4-1	\$	05 050 00
				Co	Sub-total neral Conditions	\$	85,250.00 4,262.50
				Ge	Sub-total	-	89,512.50
						=	
					Insurance	_	1,790.25
							1,790.25
					Sub-total		93,093.00
					Mark up		13,963.95
					Total	\$	107,056.95
						_	
Does Pro	posed Change involve a change in Contract Sum?	[] No	[X] Yes	[Increase / Decrease]		\$1	107,056.95
Does Pro	posed Change involve a change in Contract Time?	[] No	[X] Yes	[Increase / Decrease]	Days		10
JC Comm							
CONTRA				Architect			
	epointe Drive, Suite #129						
Lewisville	, TX 75057						
Ву:	Scott Arthur			By:			
Signatı							
•	ile. Scott Arthur			Signature:			
Date:	3/19/2020			Date:			

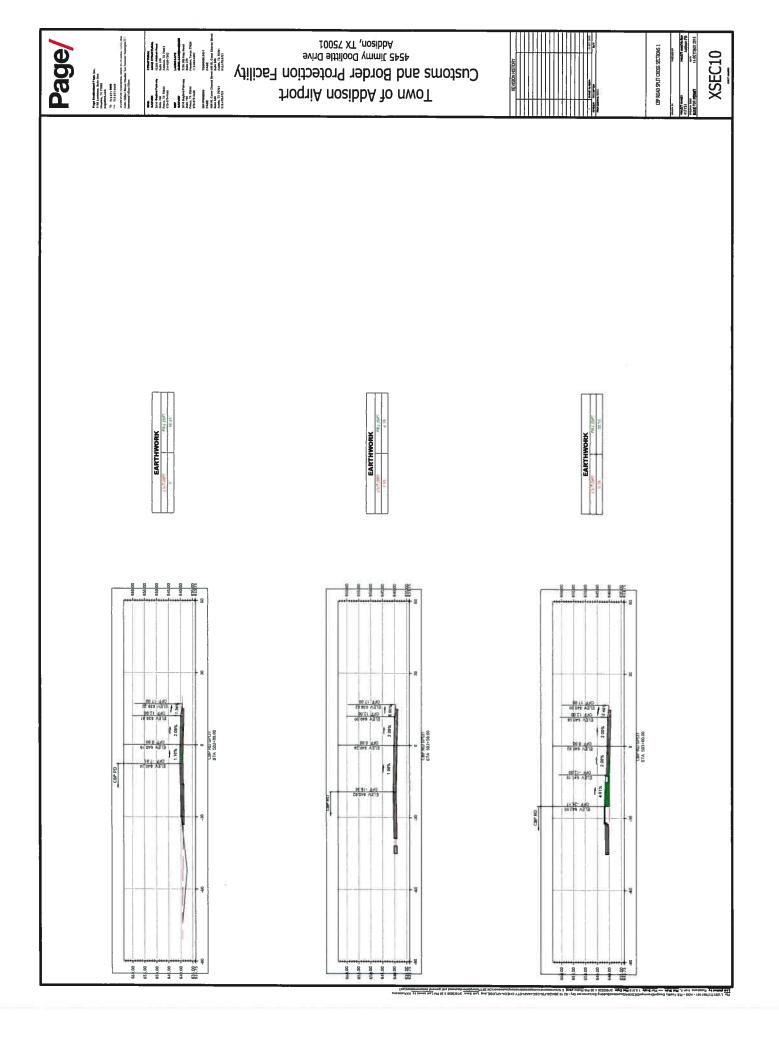
Proposal #16

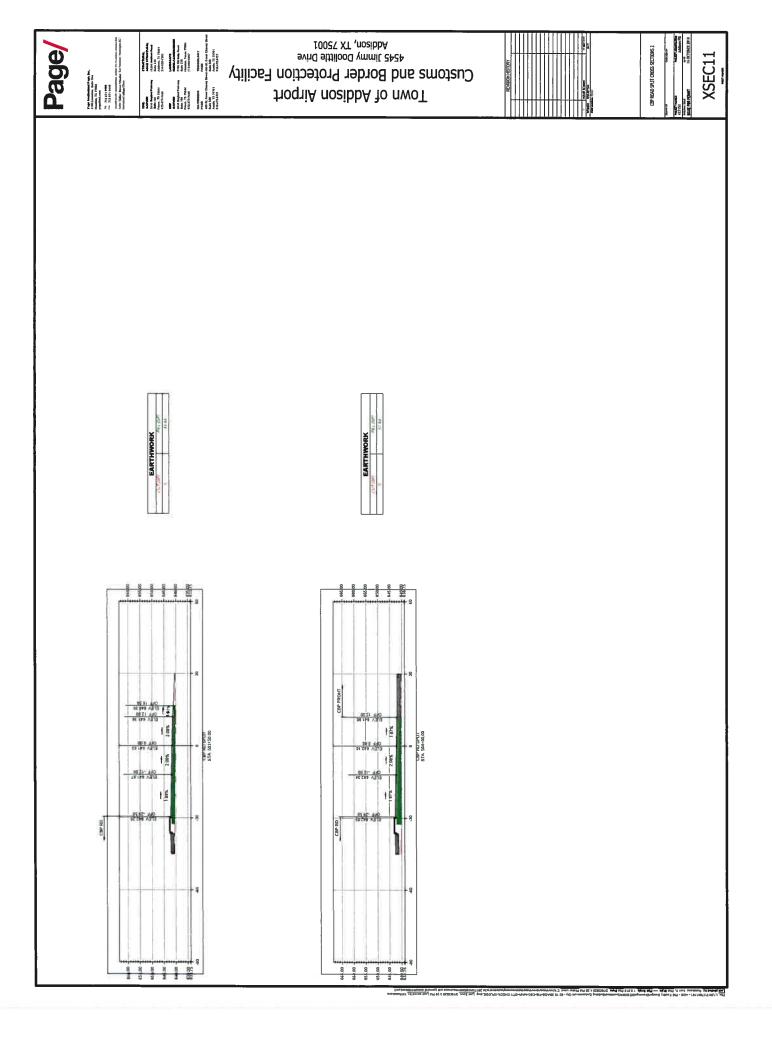


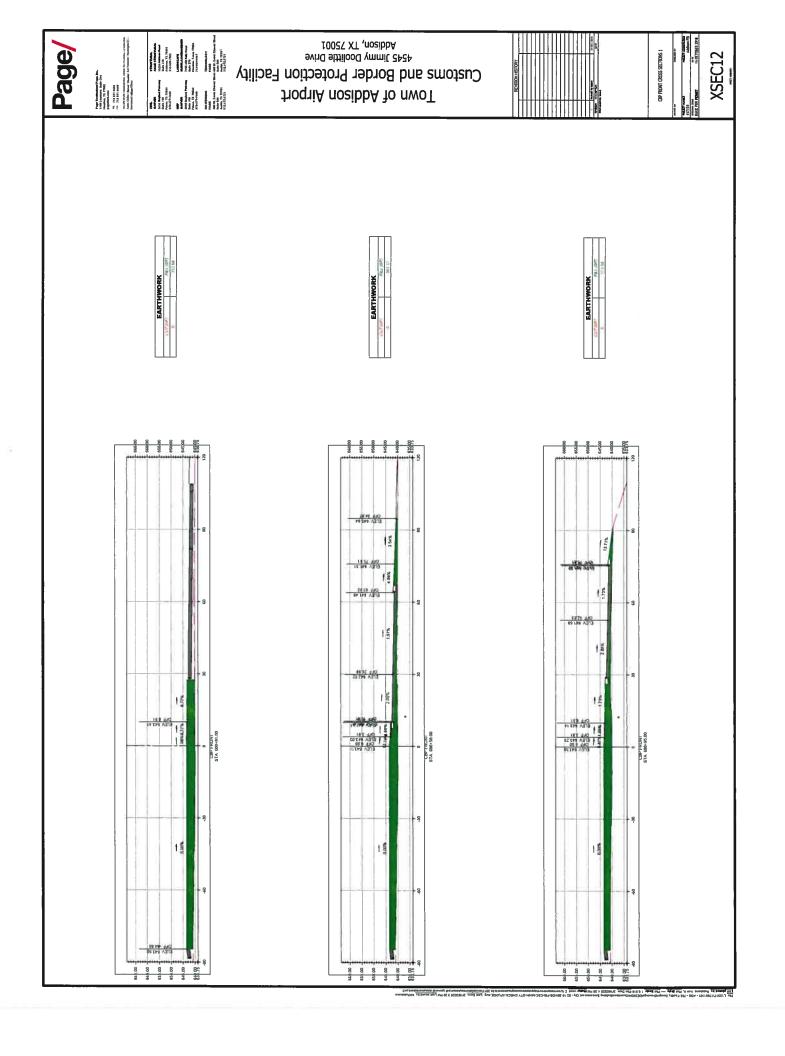


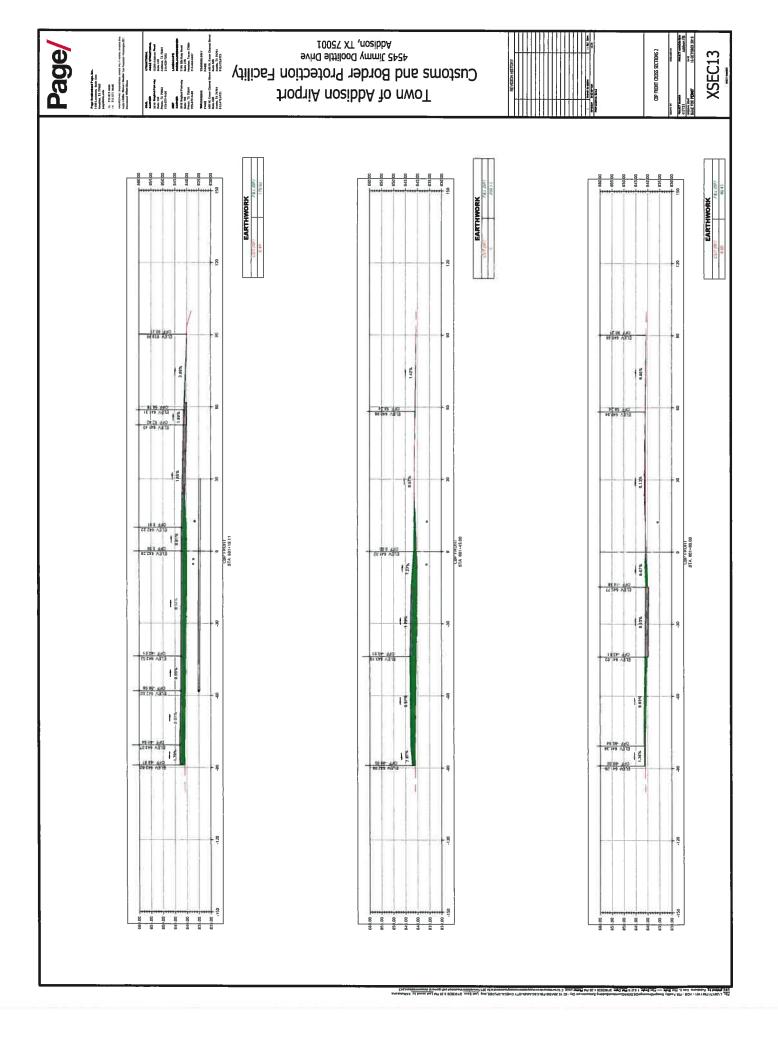












Addison Airport FIS Landside Earthwork

ANDSCAPING		Original Survey Take	off Calculations	10.5	survey Takeoff Calcula	tions
Location	Area (SF)	Average Depth (FT)	Volume (CY)	Average Depth (FT)	Volume (CY)	Volume w/Compaction (CY)
1	1752.00	1.04	67.48	1.86	120.69	168.97
2	1929.00	0.78	55.73	1.21	86.45	121.03
3	4719.00	0.38	66.42	0.04	6.99	9.79
4	2659.00	0.54	53.18	0.57	56.13	UKSH
5	2014.00	0.62	46.25	0.55	41.03	57.44
6	4393.00	0.09	14.64	0.81	131.79	184.51
7	4192.00	-0.84	-130.42	0.10	15.53	21.74
8	4069.00	-0.18	-27.13	0.12	18.08	25.32
9	4035.00	-0.28	-41.84	0.01	1.49	2.09
10	857.00	1.23	39.04	1.33	42.22	59.10
11	7647.00	-0.15	-42.48	-0.06	0.00	0.00
12	627.00	2.56	59.45	2.56	59.45	83.23
13	5828.00	0.65	140.30	0.45	97.13	135.99
14	3265.00	0.22	26.60	0.27	32.65	45.71
33	956.00	2.32	82.15	3.07	108.70	152.18
34	540.00	2.43	48.60	2.43	48.60	68.04
35	3354.00	0.39	48.45	0.72	89.44	125.22
		Subtotal =	506.41		956.38	1338.93
		Concrete Volume (CY) =	0.00		0.00	0.00
		Site Stripping (CY) =	0.00		0.00	332.68
		Total Volume (CY) =	506.41		956.38	1671.61

E LÄNE		Original Survey Takeoff Calculations		JC Survey Takeoff Calculations		
Location	Area (SF)	Average Depth (FT)	Volume (GY)	Average Depth (FT)	Volume (CY)	Volume W/Compaction (CY)
15	1344.00	2.06	102.54	2.99	148.84	208.37
16	1321.00	2.02	98.83	3.32	162.43	227.41
17	2649.00	1.98	194.26	2.18	213.88	299.44
20	2321.00	1.45	124.65	1.79	153.87	215.42
22	1939.00	1.51	108.44	1.60	114.90	160.87
24	2767.00	2.54	260.30	1.94	198.81	278.34
26	3586.00	1.48	196.57	1.51	200.55	280.77
27	4083.00	0.72	108.88	0.63	95.27	133.38
28	3953.00	0.63	92.24	0.84	122.98	172.18
29	2625.00	0.39	37.92	0.96	93.33	130.67
		Subtotal =	1324.62		1504.88	2106.83
		Concrete Volume (CY) =	-659,78		-659.78	-659.78
		Site Stripping (CY) =	0,00		0.00	246.19
		Total Volume (CY) =	664.84		845.10	1693.24

JEWALK		Original Survey Takeoff Calculations		JC Survey Takeoff Calculations		
Location	Area (SF)	Average Depth (FT)	Volume (CY)	Average Depth (FT)	Volume (GY)	Total Volume w/Compaction (CY)
30	2645.00	2.41	236.09	2.41	236.09	330.53
		Subtotal =	236.09		236.09	330.53
		Concrete Volume (CY) =	-32,33		-32.33	-32.33
		Site Stripping (CY) =	0.00		0.00	0,00
		Total Volume (CY) =	203.76		203.76	298.20

PARKING AREA		Original Survey Takeoff Calculations		JC Survey Takeoff Calculations		
Location	Area (SF)	Average Depth (FT)	Volume (CY)	Average Depth (FT)	Volume (CY)	Volümė W/Compaction (CY)
18	1431.00	1.14	60.42	1.26	66.78	93.49
19	1364.00	2.35	118.72	2.35	118.72	166.21
21	1644.00	2.09	127.26	2.17	132.13	184.98
23	545.00	2.76	55.71	2.63	53.09	74.32
25	2581.00	2.16	206.48	2.18	208.39	291.75
		Subtotal =	568.59		579.11	810.75
		Concrete Volume (CY) =	-187,73		-187.73	-187.73
		Site Stripping (CY) =	0.00		0.00	70.05
		Total Volume (CY) =	380.86		391.38	693.07

WEST FILL		Original Survey Takeoff Calculations		JC Survey Takeoff Calculations		
Location	Area (SF)	Average Depth (FT)	Volume (CY)	Average Depth (FT)	Volume (CY)	Volume W/Compaction (CY)
31	3812.00	0.00	0.00	2.69	379.79	531.70
		Subtotal =	0.00		379.79	531.70
		Concrete Volume (CY) =	0.00		0.00	0.00
		Site Stripping (CY) =	0.00		0.00	0.00
		Total Volume (CY) =	0.00		379.79	531.70

EARTHWORK VOLUME SUMMARY							
LOCATION	Original Survey Takeoff VOLUME (CY)	JC Survey Average End Area Method VOLUME (CY)	JC Survey Takeoff VOLUME (CY)	JC Survey Takeofi w/Compaction VOLUME (CY)			
LANDSCAPING =	506.41		956.38	1671.61			
FIRE LANE =	664.84		845.10	1693.24			
SIDEWALK =	203.76		203.76	298.20			
PARKING AREA =	380.86		391.38	693.07			
FOUNDATION =	0.00		0.00	1633.92			
WEST FILL =	0.00		379.79	531.70			
TOTAL =	1755.87	1957.00	2776.40	6521.74			
CONTRACT EMBA	CONTRACT EMBANKMENT QUANTITY (CY) =		1006,00	1006.00			
DIFFE	RENCE IN VOLUME (CY) =	951.00	1770.40	5515.74			

ITEM P-152 EXCAVATION, SUBGRADE, AND EMBANKMENT

DESCRIPTION

152-1.1 This item covers excavation, disposal, placement, and compaction of all materials within the limits of the work required to construct safety areas, runways, taxiways, aprons, and intermediate areas as well as other areas for drainage, building construction, parking, or other purposes in accordance with these specifications and in conformity to the dimensions and typical sections shown on the plans.

152-1.2 CLASSIFICATION. All material excavated shall be classified as defined below:

- a. Unclassified Excavation. Unclassified excavation shall consist of the excavation and disposal of all material, regardless of its nature which is not otherwise classified and paid for under one of the following items.
- b. Rock Excavation. Rock excavation shall include all solid rock in ledges, in bedded deposits, in unstratified masses, and conglomerate deposits which are so firmly cemented they cannot be removed without blasting or using rippers. All boulders containing a volume of more than 1/2 cubic yard will be classified as "rock excavation."
- of soils and organic matter not suitable for foundation material. Muck shall include materials that will decay or produce subsidence in the embankment. It may consist of decaying stumps, roots, logs, humus, or other material not satisfactory for incorporation in the embankment.
- d. Drainage Excavation. Drainage excavation shall consist of all excavation made for the primary purpose of drainage and includes drainage ditches, such as intercepting, inlet or outlet ditches; temporary levee construction; or any other type as shown on the plans.
- e. Borrow Excavation. Borrow excavation shall consist of approved material required for the construction of embankments or for other portions of the work in excess of the quantity of usable material available from required excavations. Borrow material shall be obtained from areas designated by the Engineer within the limits of the airport property but outside the normal limits of necessary grading, or from areas outside the airport boundaries.
- 152-1.3 Unsuitable Excavation. Any material containing vegetable or organic matter, such as muck, peat, organic silt, or sod shall be considered unsuitable for use in embankment construction. Material, suitable for topsoil may be used on the embankment slope when approved by the Engineer. Material not considered by the Engineer to be suitable for use on the embankment slope shall be disposed of off-site or as directed by the Engineer. Undercutting of material unsatisfactory for subgrade foundation, roads, shoulders, or areas intended for turfing shall be considered unsuitable excavation and shall be excavated to the depth specified by the Engineer below the subgrade.

CONSTRUCTION METHODS

152-2.1 General. Before beginning excavation, grading, and embankment operations in any area, the area shall be completely cleared and grubbed in accordance with Item P-151.

The suitability of material to be placed in embankments shall be subject to approval by the Engineer. All unsuitable material shall be disposed of in waste areas shown on the plans. All waste areas shall be graded to allow positive drainage of the area and of adjacent areas. The surface elevation of waste areas shall not extend above the surface elevation of adjacent usable areas of the airport, unless specified on the plans or approved by the Engineer.

When the Contractor's excavating operations encounter artifacts of historical or archaeological significance, the operations shall be temporarily discontinued and the Engineer notified per subsection 70-20 of the General Provisions. At the direction of the Engineer, the Contractor shall excavate the site in such a manner as to preserve the artifacts encountered and allow for their removal. Such excavation will be paid for as extra work.

Those areas outside of the limits of the pavement areas where the top layer of soil material has become compacted by hauling or other Contractor activities shall be scarified and disked to a depth of 4 inches, to loosen and pulverize the soil.

If it is necessary to interrupt existing surface drainage, sewers or under-drainage, conduits, utilities, or similar underground structures, the Contractor shall be responsible for and shall take all necessary precautions to preserve them or provide temporary services. When such facilities are encountered, the Contractor shall notify the Engineer, who shall arrange for their removal if necessary. The Contractor, at his or her expense, shall satisfactorily repair or pay the cost of all damage to such facilities or structures that may result from any of the Contractor's operations during the period of the contract.

152-2.2 EXCAVATION. No excavation shall be started until the work has been staked out by the Contractor and the Engineer has obtained from the Contractor the survey notes of the elevations and measurements of the ground surface. All areas to be excavated shall be stripped of vegetation and topsoil. Topsoil shall be stockpiled for future use in areas designated on the plans or by the Engineer. All suitable excavated material shall be used in the formation of embankment, subgrade, or other purposes shown on the plans. All unsuitable material shall be disposed of as *described in paragraph 152-1.3* shown on the plans.

When the volume of the excavation exceeds that required to construct the embankments to the grades indicated, the excess shall be used to grade the areas of ultimate development or disposed as directed by the Owner. When the volume of excavation is not sufficient for constructing the embankments to the grades indicated, the deficiency shall be obtained from borrow areas.

The grade shall be maintained so that the surface is well drained at all times. When necessary, temporary drains and drainage ditches shall be installed to intercept or divert surface water that may affect the work.

- **a. Selective Grading.** When the quality of material varies selective grading is indicated on the plans, the more suitable material designated by the Engineer shall be used in constructing the embankment or in capping the pavement subgrade. If, at the time of excavation, it is not possible to place this material in its final location, it shall be stockpiled in approved areas. so that it can be measured for payment as specified in paragraph 152 3.3. . Selective grading will not be measured for separate payment but will be considered subsidiary to "Unclassified Excavation".
- **b. Undercutting.** Rock, shale, hardpan, loose rock, boulders, or other material unsatisfactory for safety areas, subgrades, roads, shoulders, or any areas intended for turf shall be excavated to a minimum depth of 12 inches below the subgrade or to the depth specified by the Engineer. Muck, peat, matted roots, or other yielding material, unsatisfactory for subgrade foundation, shall be removed to the depth specified. Unsuitable materials shall be disposed off the airport. The cost is incidental to this item *disposed of as directed in paragraph 152-1.3*. This excavated material shall be paid for at the contract unit price per cubic yard for **Unsuitable Excavation**. The excavated area shall be backfilled with suitable material obtained from the grading operations or borrow areas and compacted to specified densities. The necessary backfill will constitute a *necessary part of Unsuitable Excavation* part of the embankment. Where rock cuts are made, backfill with select material. Any pockets created in the rock surface shall be drained as directed by the Engineer in accordance with the details shown on the plans.
- c. Overbreak. Overbreak, including slides, is that portion of any material displaced or loosened beyond the finished work as planned or authorized by the Engineer. All overbreak shall be graded or removed by the Contractor and disposed of as directed by the Engineer. The Engineer shall determine if the displacement of such material was unavoidable and his or her decision shall be final. Payment will not be

made for the removal and disposal of overbreak that the Engineer determines as avoidable. Unavoidable overbreak will be classified as "Unclassified Excavation."

- **d. Removal of Utilities.** The removal of *some* existing structures and utilities required to permit the orderly progress of work *may* will be accomplished by someone other than the Contractor; for example, the utility unless otherwise shown on the plans. All existing foundations shall be excavated at least 2 feet below the top of subgrade or as indicated on the plans, and the material disposed of as directed by the Engineer. All foundations thus excavated shall be backfilled with suitable material and compacted as specified. *All work associated with the excavation, removal, backfill, disposal, and/or stockpiling of existing structures and culverts be considered subsidiary to "Unclassified Excavation".*
- **e. Compaction Requirements.** The subgrade under areas to be paved shall be compacted to a depth of 8" and to a density of not less than 95 percent of the maximum density as determined by ASTM D1557. The material to be compacted shall be within ±2% of optimum moisture content before being rolled to obtain the prescribed compaction (except for expansive soils).

The in-place field density shall be determined in accordance with ASTM D6938 using Procedure A, the direct transmission method, and ASTM D6938 shall be used to determine the moisture content of the material. The machine shall be calibrated in accordance with ASTM D6938. Stones or rock fragments larger than 4 inches in their greatest dimension will not be permitted in the top 6 inches of the subgrade. The finished grading operations, conforming to the typical cross-section, shall be completed and maintained at least 1,000 feet ahead of the paving operations or as directed by the Engineer.

All loose or protruding rocks on the back slopes of cuts shall be pried loose or otherwise removed to the slope finished grade line. All cut-and-fill slopes shall be uniformly dressed to the slope, cross-section, and alignment shown on the plans or as directed by the Engineer.

Blasting shall not be allowed.

- **f. Proof Rolling.** After compaction is completed, the subgrade area shall be proof rolled with heavy pneumatic-tired roller having four or more tires abreast, each tire loaded to a minimum of 30,000 pounds and inflated to a minimum of 125 psi in the presence of the independent QC Engineer. Apply a minimum of 50% coverage, or as specified by the Engineer, to all paved areas. A coverage is defined as the application of one tire print over the designated area. Soft areas of subgrade that deflect more than 1 inch or show permanent deformation greater than 1 inch shall be removed and replaced with suitable material or reworked to conform to the moisture content and compaction requirements in accordance with these specifications.
- **152-2.3 BORROW EXCAVATION**. Borrow areas within the airport property are indicated on the plans. Borrow excavation shall be made only at these designated locations and within the horizontal and vertical limits as staked or as directed by the Engineer.

When borrow sources are outside the boundaries of the airport property, it shall be the Contractor's responsibility to locate and obtain the borrow sources, subject to the approval of the Engineer. The Contractor shall notify the Engineer at least 15 days prior to beginning the excavation so necessary measurements and tests can be made. All borrow pits shall be opened up to expose the various strata of acceptable material to allow obtaining a uniform product. All unsuitable material shall be disposed of by the Contractor. Borrow pits shall be excavated to regular lines to permit accurate measurements, and they shall be drained and left in a neat, presentable condition with all slopes dressed uniformly.

152-2.4 DRAINAGE EXCAVATION. Drainage excavation shall consist of excavating for drainage ditches such as intercepting; inlet or outlet ditches; for temporary levee construction; or for any other type as designed or as shown on the plans. The work shall be performed in sequence with the other construction. Intercepting ditches shall be constructed prior to starting adjacent excavation operations. All satisfactory material shall be placed in embankment fills; unsuitable material shall be placed in designated waste areas

or as directed by the Engineer. All necessary work shall be performed true to final line, elevation, and cross-section. The Contractor shall maintain ditches constructed on the project to the required cross-section and shall keep them free of debris or obstructions until the project is accepted.

152-2.5 PREPARATION OF EMBANKMENT AREA. Where an embankment is to be constructed to a height of 4 feet or less, all sod and vegetative matter shall be removed from the surface upon which the embankment is to be placed. The cleared surface shall be broken up by plowing or scarifying to a minimum depth of 6 inches and shall then be compacted as indicated in paragraph 152-2.6.

When the height of fill is greater than 4 feet, sod not required to be removed shall be thoroughly disked and recompacted to the density of the surrounding ground before construction of embankment.

Sloped surfaces steeper than one (1) vertical to four (4) horizontal shall be plowed, stepped, benched, or broken up so that the fill material will bond with the existing material. When the subgrade is part fill and part excavation or natural ground, the excavated or natural ground portion shall be scarified to a depth of 12 inches and compacted as specified for the adjacent fill.

No direct payment shall be made for the work performed under this section. The necessary clearing and grubbing and the quantity of excavation removed will be paid for under the respective items of work.

152-2.6 FORMATION OF EMBANKMENTS. Embankments shall be formed in successive horizontal layers of not more than 8 inches in loose depth for the full width of the cross-section, unless otherwise approved by the Engineer.

The layers shall be placed, to produce a soil structure as shown on the typical cross-section or as directed by the Engineer. Materials such as brush, hedge, roots, stumps, grass and other organic matter, shall not be incorporated or buried in the embankment.

Earthwork operations shall be suspended at any time when satisfactory results cannot be obtained because of rain, freezing, or other unsatisfactory weather conditions in the field. Frozen material shall not be placed in the embankment nor shall embankment be placed upon frozen material. Material shall not be placed on surfaces that are muddy, frozen, or contain frost. The Contractor shall drag, blade, or slope the embankment to provide surface drainage at all times.

The material in each layer shall be within ±2% of optimum moisture content before rolling to obtain the prescribed compaction. To achieve a uniform moisture content throughout the layer, the material shall be moistened or aerated as necessary. Samples of all embankment materials for testing, both before and after placement and compaction, will be taken for each 1000SY of material placed. Based on these tests, the Contractor shall make the necessary corrections and adjustments in methods, materials or moisture content to achieve the specified embankment density.

Rolling operations shall be continued until the embankment is compacted to not less than 95% of maximum density for noncohesive soils, and 90% of maximum density for cohesive soils as determined by ASTM D1557. Under all areas to be paved, the embankments shall be compacted to a depth of 8" and to a density of not less than 95 percent of the maximum density as determined by ASTM D1557.

On all areas outside of the pavement areas, no compaction will be required on the top 4 inches.

The in-place field density shall be determined in accordance with ASTM D6938 using Procedure A, the direct transmission method, and ASTM D6938 shall be used to determine the moisture content of the material. The machine shall be calibrated in accordance with ASTM D6938. The Engineer shall perform all density tests.

Compaction areas shall be kept separate, and no layer shall be covered by another layer until the proper density is obtained.

During construction of the embankment, the Contractor shall route all construction equipment evenly over the entire width of the embankment as each layer is placed. Layer placement shall begin in the deepest portion of the embankment fill. As placement progresses, the layers shall be constructed approximately parallel to the finished pavement grade line.

When rock and other embankment material are excavated at approximately the same time, the rock shall be incorporated into the outer portion of the embankment and the other material shall be incorporated under the future paved areas. Stones or fragmentary rock larger than 4 inches in their greatest dimensions will not be allowed in the top 6 inches of the subgrade. Rockfill shall be brought up in layers as specified or as directed by the Engineer and the finer material shall be used to fill the voids with forming a dense, compact mass. Rock or boulders shall not be disposed of outside the excavation or embankment areas, except at places and in the manner designated on the plans or by the Engineer.

When the excavated material consists predominantly of rock fragments of such size that the material cannot be placed in layers of the prescribed thickness without crushing, pulverizing or further breaking down the pieces, such material may be placed in the embankment as directed in layers not exceeding 2 feet in thickness. Each layer shall be leveled and smoothed with suitable equipment by distribution of spalls and finer fragments of rock. The layer shall not be constructed above an elevation 4 feet below the finished subgrade.

There will be no separate measurement of payment for compacted embankment. All costs incidental to placing in layers, compacting, discing, watering, mixing, sloping, and other operations necessary for construction of embankments will be included in the contract price for excavation, borrow, or other items.

Imported material for fill or backfill under pavements shall be free of rock fragments greater than 4 inches in size, organic matter, and other deleterious materials. The material should also have a liquid limit less than 60 and plasticity index at least 16. The fine content (percent passing a No. 200 sieve) of the material should be 45 percent or more.

152-2.7 FINISHING AND PROTECTION OF SUBGRADE. After the subgrade is substantially complete, the Contractor shall remove any soft or other unstable material over the full width of the subgrade that will not compact properly. All low areas, holes or depressions in the subgrade shall be brought to grade with suitable select material. Scarifying, blading, rolling and other methods shall be performed to provide a thoroughly compacted subgrade shaped to the lines and grades shown on the plans.

Grading of the subgrade shall be performed so that it will drain readily. The Contractor shall protect the subgrade from damage and limit hauling over the finished subgrade to only traffic essential for construction purposes. All ruts or rough places that develop in the completed subgrade shall be graded and recompacted.

No subbase, base, or surface course shall be placed on the subgrade until the subgrade has been approved by the Engineer.

152-2.8 HAUL. All hauling will be considered a necessary and incidental part of the work. The Contractor shall include the cost in the contract unit price for the pay of items of work involved. No payment will be made separately or directly for hauling on any part of the work.

152-2.9 TOLERANCES. In those areas upon which a subbase or base course is to be placed, the top of the subgrade shall be of such smoothness that, when tested with a 12-foot straightedge applied parallel and at right angles to the centerline, it shall not show any deviation in excess of 1/2 inch, or shall not be more than 0.05 feet from true grade as established by grade hubs. Any deviation in excess of these amounts shall be corrected by loosening, adding, or removing materials; reshaping; and recompacting.

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On safety areas, intermediate and other designated areas, the surface shall be of such smoothness that it will not vary more than 0.10 feet from true grade as established by grade hubs. Any deviation in excess of this amount shall be corrected by loosening, adding or removing materials, and reshaping.

152-2.10 TOPSOIL. When topsoil is specified or required as shown on the plans or under Item T-905, it shall be salvaged from stripping or other grading operations. The topsoil shall meet the requirements of Item T-905. If, at the time of excavation or stripping, the topsoil cannot be placed in its final section of finished construction, the material shall be stockpiled at approved locations. Stockpiles shall not be placed within **400** feet of runway pavement or **100** feet of taxiway pavement and shall not be placed on areas that subsequently will require any excavation or embankment fill. If, in the judgment of the Engineer, it is practical to place the salvaged topsoil at the time of excavation or stripping, the material shall be placed in its final position without stockpiling or further rehandling.

Upon completion of grading operations, stockpiled topsoil shall be handled and placed as directed, or as required in Item T-905.

No direct payment will be made for topsoil under Item P-152. The quantity removed and placed directly or stockpiled shall be paid for at the contract unit price per cubic yard for "Unclassified Excavation."

When stockpiling of topsoil and later rehandling of such material is directed by the Engineer, the material so rehandled shall be paid for at the contract unit price per cubic yard for "topsoiling," as provided in Item T-905.

METHOD OF MEASUREMENT

152-3.1 The quantity of excavation to be paid for shall be the number of cubic yards measured in its original position. Measurement shall not include the quantity of materials excavated without authorization beyond normal slope lines, or the quantity of material used for purposes other than those directed.

Measurement of excavation/embankment shall be based on plan quantities. These quantities are believed to be correct and shall be utilized for final excavation quantity payment not withstanding any adjustments to the project by written direction of the Engineer. Should the contractor find discrepancies and/or errors, he/she shall bring the discrepancy and/or error to the attention of the Engineer immediately and corrections shall be made to the quantity of excavation to be paid for by change order. It is expressly understood by the contractor that upon disturbance of the existing ground and no notification to the engineer of possible errors, that the contractor accepts as final payment the quantities of excavation as detailed on the plans and laid out in the proposal. No adjustment has been made to the plan quantities for the construction or demolition of existing drainage structures. The Contractor shall make his own determination as to the amount of unsuitable excavated material which may be encountered and the resulting additional borrow material required for the construction of the embankment. There will be no adjustment for additional embankment required to construct the project if the excavated material is deemed unsuitable.

- 152-3.2 Borrow material shall be paid for on the basis of the number of cubic yards measured in its original position at the borrow pit.
- 152-3.3 Stockpiled material shall be paid for on the basis of the number of cubic yards measured in the stockpiled position as soon as the material has been stockpiled.
- 152-3.2 For payment specified by the cubic yard, measurement for all excavation/embankment shall be computed by the average end area method. The end area is that bound by the original ground line established by field cross-sections and the final theoretical pay line established by Excavation/embankment cross-sections shown on the plans, subject to verification by the Engineer. After completion of all excavation/embankment operations and prior to the placing of base or subbase material, the final excavation/embankment shall be verified by the Engineer by means of field cross-sections taken randomly at intervals not exceeding 500 linear feet.

In cut sections, the additional cut required to construct the topsoil layer to the plan grade has not been measured and will not be measured for separate payment but will be subsidiary to "Unclassified Excavation". In fill sections, the additional fill required to replace the stripped material has not been measured and will not be measured for payment but will be subsidiary to "Unclassified Excavation".

No allowance has been made in the measurement for shrink/swell. The Contractor shall make his own determination as to the amount of shrink/swell involved in the construction of the embankment.

The Contractor shall make his own determination as to the suitability of the excavated material to be placed in embankments and the resulting additional off-site material required for the construction of the embankment. Additional off-site material required for the formation of embankment shall not be measured for separate payment but shall be considered subsidiary to "Unclassified Excavation".

152-3.6 Unsuitable excavation shall be measured from the surface of the ground, after stripping has been accomplished, or from the bottom of the planned excavation, to the depth of the excavation as directed by the Engineer. Measurements will be taken by the Engineer, and the volume of excavation will be calculated by the average end area method. The necessary refilling of unsuitable areas will not be measured for separate payment but will be subsidiary to "Unsuitable Excavation". Only that amount of excavation directed by the Engineer will be measured for payment.

BASIS OF PAYMENT

152-4.1 "Unclassified excavation" payment shall be made at the contract unit price per cubic yard. This price shall be full compensation for furnishing all materials, labor, equipment, tools, and incidentals necessary to complete the item.

Payment will be made under:

Item P-152-4.1	Unclassified Excavation – per Cubic Yard
Item P-152-4.2	Embankment – per Cubic Yard

TESTING REQUIREMENTS

ASTM D698	Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³)
ASTM D1556	Standard Test Method for Density and Unit Weight of Soil in Place by the Sand-Cone Method
ASTM D1557	Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft³)
ASTM D2167	Standard Test Method for Density and Unit Weight of Soil in Place by the Rubber Balloon Method
ASTM D6938	Standard Test Methods for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth)

END OF ITEM P-152

Stevanovic, Vladimir (Vlad)

From: Bunch, Kyle <kbunch@mooresupply.com>

Sent: Monday, April 13, 2020 9:28 AM

To: Juan Tobias **Subject:** Fw: Acorn

Kyle Bunch Moore Supply Company

Phone:817-784-2340 Fax:817-784-2335 E-fax:484-708-1921

Sent: Monday, April 13, 2020 8:56 AM

To: Bunch, Kyle <kbunch@mooresupply.com>

Subject: Acorn

NOTE: THIS IS AN EXTERNAL EMAIL

Kyle,

Acorn orders are non returnable and sometimes non cancable once sent off into production since these are made to order based on signed submittals with desired options.

Blake LenamondDillard Associates

214.843.2218 cell

1504 Selene Dr, Carrollton TX 75006 (972.278.2000) 6100 Tri County Parkway, Schertz TX 78154 (210.824.4881)



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N C	ORPORATED		Proposal #17	
Project:	Addison Airport Customs and	Customer:	Town of Addison	_
	Border Protection Facility	_	5350 Beltline Road	
Date:	4/14/2020		Dallas, Texas 75254	_
Attn:	Jeff Mechlem, Mitchell McAnally	Architect:	Page Southerland Page	
	Will Butler, Vlad Stevanovic		1100 Louisiana Street, Suite One	
_		- 	Houston, Texas 77002	

This Proposal contains an itemized quotation for changes in the Contract Sum and/or Contract Time in response to proposed modifications to the Contract Documents based on RFI 89.

Item	Description		Total
1	Install and provide new WC/Lavatory combo unit per RFI 89.	\$	2,620.00
		_	
	Sub-total	\$	2,620.00
	General Conditions	\$	131.00
	Sub-total	\$	2,751.00
	Insurance	\$	55.02
	Bond	\$	55.02
	Sub-total	\$	2,861.04
	Mark up	\$	429.16
	Total	\$	3,290.20

Does Proposed Change involve a change in Contract Sum?	[] No	[X] Yes	[Increase / Decrease]		\$3,290.20
Does Proposed Change involve a change in Contract Time?	[] No	[X] Yes	[Increase / Decrease]	Days	10

JC Commercial	Page Southerland Page
CONTRACTOR	Architect
1801 Lakepointe Drive, Suite #129	1100 Louisiana Street, Suite One

1001 Editopolitic Dive, Gaile #120

Lewisville, TX 75057

Scott Arthur

Signature:

Scott Arthur

Date:

By:

4/14/2020

By:

Signature:

Houston, Texas 77002

Date:



1005 N Henderson Street Fort Worth, TX 76107 817.900.2030

Change Order

Date: 4-13-20

Job: Customs and Border Protection Facility

To: ESTIMATING

Plumbing bid per RFI 89 and specs. Including the following fixtures:

- Install and provide new WC/Lavatory combo unit
- Change fixture type from fixture schedule to RFI 89
- 6-8 week lead time from the factory (non-stock item)
- · Pricing based on normal work hours M-F, no expedited schedule
- MEP ceiling coordination meeting will be required prior to overhead piping
- Fixtures to be per schedule or equal.

Base Bid Plumbing \$ 2,620.00

No tax included

Exclusions;

No HVAC condensate. No Gas. NO BIM or ENGINEERED DRAWINGS. No rock excavation. No soil retainage/void systems. No taps, meters or fees. No coring, saw cut or XRAY in post tension slab. No saw cut or pour back. No pour back of existing leave outs. No painting of any pipe. No spoil haul off. No low voltage wiring if required. No sub-meter unless noted above, No backflow unless noted above. No DEMO or REPLACEMENT of ceilings, walls, wall coverings, floors, flooring, cabinets or any other item covering or encasing any plumbing to tie into or demo.

Thank You, Juan Tobias M-22558

Texas State Board of Plumbing Examiners 929 E, 41st Austin, Tx 78751 512-936-5200



Penal-Ware® 1415 Series

15" Lav-Toilet Comby with Lavatory Multi-Sided Bowl



1415-CT-2-BP-04-FV

1415-AL-2-BP-04-FV-PHRF



Fixture May Show Some Available Options

Please visit www.acorneng.com for most current specifications.

15" Lav-Toilet Comby with Lavatory Multi-Sided Bowl

Fixture is arranged to be installed on finished wall and serviced from an accessible pipe chase. Optional Wall Sleeve or Metal Template is recommended for all installations for required wall openings. Fixture is fabricated from 14 gage, type 304 stainless steel cabinet and toilet bowl and is seamless welded construction with a satin finish. The inside of the toilet bowl also has a satin finish. Cabinet interior is sound-deadened with fire-resistant material. There are no accessible voids or crevices where contraband can be concealed.

Lavatory Multi-Sided Bowl is 12-3/4" x 8-1/4" x 5" deep. Standard elbow waste outlet is 1-1/2" O.D. plain

Optional Valve may be an Air-Control pneumatically operated, pushbutton valve using atmospheric air. Pushbutton is vandal-resistant and requires less than 5 pounds to activate valve, Valve is direct acting, nonmetering type and is optionally available as metering with non-hold open feature. Metering valve timing is adjustable from 5 to 60 seconds. Valve includes a 0.5 GPM flow control and can be remotely located up to 10 feet from the operating pushbutton. Valve and bubbler conform with lead free requirements for NSF61, Section 9 and

Toilet is blowout jet type with elongated bowl manufactured to ASME A112.19.3 and CSA B45.4 requirements and will flush with a minimum of 25 PSI flow pressure when used in conjunction with a minimum of 1.28 gpf. Trap has a minimum 3-1/2" seal and will pass a 2-1/8" ball. Toilet waste outlet is 2-3/8" diameter plain end extending 3" beyond the fixture for wall outlet or Gasketed Waste for floor outlet.

Regularly Furnished items include a fast drain, integral raised soap ribs, and mounting hardware. **GUIDE SPECIFICATION**

Provide and install Acorn Penal-Ware® 15" wide Lav-Toilet Comby (specify model number and options). Fixture shall be fabricated of type 304 stainless steel. Construction shall be seamless welded and exposed surfaces shall have a satin finish. Countertop shall have raised soap ribs. Provide Air-control pneumatically operated pushbutton valve. Valve and bubbler conform with lead free requirements for NSF61, Section 9 and CHSC 116875. Toilet shall be concealed blowout jet type with an elongated bowl, a self-draining flushing rim, and an integral contoured seat. Toilet shall meet ASME A112.19.3 and CSA B45.4 requirements and will flush with a minimum of 25 PSI flow pressure when used in conjunction with a minimum of 1.28 gpf. Tollet trap shall have a minimum 3-1/2" seal that shall pass a 2-1/8" diameter ball and be fully enclosed. Cabinet interior is sound-deadened with fire-resistant material. Fixture shall withstand loadings of 5,000 pounds without permanent damage. Fixture shall be furnished with necessary fasteners for proper installation.

> Page 1 P.1415 Revised: 03/15/19

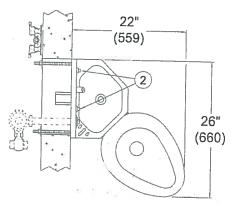
Penal-Ware® 1415: 15" Lav-Toilet Comby with Lavatory Multi-Sided Bowl

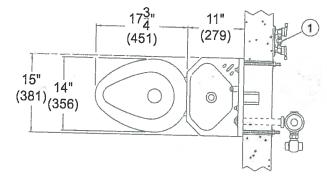


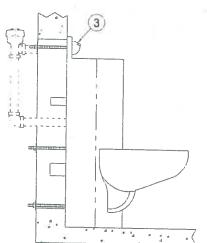
NOTES:

- 1. -04 Hot & Cold Air-Control Valve Shown
- 2. Lavy Valve Pushbuttons
- 3. Lavy -BP Penal Bubbler Shown
- 4. Lavy Waste Outlet

- 5. Optional -FV Flush Valve Shown
- 6. Toilet Waste Outlet
- 7. Wall Mounting Hardware







4" (76) $32\frac{1}{2}"$ (826) 15" $2\frac{7}{8}$ (73) 7

1415-AL-2-BP-04 SHOWN (-AR OPPOSITE)

1415-CT-1-BP-04

Penal-W	/are" 1415: 15'	' Lav-Toilet (Comb	y with	Lavatory Multi-Sided Bowl
Thickness:	Type: Concrete	Specify)		FLUSH VA	ALVE GPF's (Must Specify) PF (HET)
MODEL AND BASE MODE 1415	OPTIONS SELECTION: L NUMBER 15" Lav-Toilet Comby			☐ -1.6 GP ☐ -3.5 GP	F (Not Available in California) F (Not Available in California)
	ENTATION (Must Specif Angled Left Angled Right Centered Tollet	ήγ)		U -EVSFV	ALVE OPTIONS (Must Specify) Master-Trol® Electronic Flush Valve V Master-Trol® Electronic Flush Valve w/ Piezo Button Flush Valve, Mechanical Flush Valve by Others
FIXTURE MO -1 -2 -3	UNTING AND WASTE (I Off-Floor, Wall Outlet On-Floor, Wall Outlet On-Floor, Floor Outlet	Must Specify)		☐ -FVH ☐ -MTPFV ☐ -MTPPF\	Flush Valve, Hydraulic Master-Trol® PLUS Electronic Flush Valve / Master-Troi® PLUS Electronic Flush Valve w/ Piezo Button
BUBBLER SE -BC -BP	LECTION (Must Specify Bubbler, Code Bubbler, Penal)		CABINET -FMT	Time-Trol® Electronic Flush Valve OPTIONS Fixture Mounted Trim
☐ -BPH ☐ -LF	Bubbler, Penal Hemisphe Lav-Filler			O -IS D -PH	Integral Shelf Paper Holder, w/ -CT Centered Toilet L C R Paper Holder w/ -AL / -AR Angled Toilet
VALVE SELE Air-Control (☐ -03 ☐ -03-M ☐ -04 ☐ -04-M ☐ -04-MH	CTION (Must Specify) Pneumatic) Single Temp, Non-Meteri Single Temp, Metering Hot & Cold, Non-Meterin Hot & Cold, Metering Hot & Cold, Metering Hot & Cold, Metering	ing g		□ -TB □ -TG □ -TH □ -VG1 □ -VG2	U Left Front ☐ Right Front Toothbrush Holder ☐ Left ☐ Right 12 Gage Cabinet Towel Hook ☐ Single ☐ Double ☐ Left ☐ Right Vent Grille 5-3/4" x 8-3/4" Vent Grille 8-3/4" x 13" (Bottom Only) (-1 Only)
Master-Trol [®] ☐ -EVS1 ☐ -EVS2 ☐ -EVSP1 ☐ -EVSP2		n		LAVATORY -CW -LW1 -LWE	WASTE OPTIONS Combined Waste Lavy Thru-Wall Extension With P-Trap Lavy Waste Extension (3" Standard) Specify Length Beyond Fixture: Lavatory Overflow
Master-Trol® -MTP1 -MTP2 -MTPP1 -MTPP2 -MTP VALV -PFB	PLUS (Electronic) Single Temp Hot & Cold Single Temp, Piezo Butto Hot & Cold, Piezo Button: E OPTION Power Fallure Bypass (Pri in the event of power fail	s ovides drinking wal		☐ -PT TOILET OF ☐ -CO1	Cleanout w/ 2-3/8" OD O-Ring Connection to No-Hub 4" (Plain End Only, Not Applicable with -GW or -3 Floor Outlet) Cleanout w/ 2-3/8" OD O-Ring Connection to No-Hub 3" (Plain End Only, Not Applicable with GW or -3 Floo Outlet)
_] -MVC2	Single Temp Single Temp Single Temp Battery Pow (Batteries Not Included) Hot & Cold Hot & Cold Battery Power (Batteries Not Included)			☐ -COH ☐ -FT ☐ -FTA ☐ -FTE ☐ -FVT ☐ -FVO ☐ -GW	Cleanout Hook Assembly Flood-Trol (Manual Reset) Flood-Trol Auto-Reset Flood-Trol Electronic Flush Valve Thru Wall Connector Flush Valve Opposite In Lieu Of Standard Location Gasketed Toilet Waste (Wall Outlet)
] -PPZ1	e (Electronic) w/9VDC P Single Temp Programmab Hot & Cold Programmable	ole Piezo Button		□ -HPS□ -HS□ -PC□ -SPS□ -TSC	High Polished Seat Hinged Seat Pinned Cleanout Plug (For -CO1 Options above) Ligature Resistant Skirt
Valve By Othe	Punched for Valve by Oth	ers		TWE	Toilet Shipping Cover Toilet Waste Extension (3" Standard) Specify Length Beyond Fixture:
_] -MA2 _] -MA3	Brass Body Valve Cycle Interrupt for Time- Manifolded, 2-Stack Manifolded, 3-Stack (N/A	with -PFB option)	(PRODUCT (2-3/8" P-Trap w/ 3" Plain End Waste Outlet DPTIONS Enviro-Glaze Color Specify: Toilet Interior & Exterior Enviro-Glaze Color Specify:
] -PBH] -PBP	Manifolded, 4-Stack (N/A Hemispherical Pushbutto Pushrod Activated Pushb Transformer, 120VAC to 7 (-MVC option only)	n utton 24VAC	(☐ -LPFV ☐ -MT ☐ -SW	Toilet Exterior Only Less Punching for Flush Valve Metal Template (Only 1 required per project) Wall Sleeve AcornVac Systems
	.acorneng.com for most cu		∆ W	ARNING: Can	cer and Reproductive Harm - Mary B65Washings on
Dimensions are subject					re. Do not rough in without certified dimensions. for use of void or superseded data. © Copyright 2009 Acom Engineering Company
Model No. &	Selection Summary Option				Approved for Manufacturing
Quantity					Title
-Zeomity				Signature	Date
		Page 3	P.141	5	Revised: 03/15/19